

- Introduction
- Prevention through Design
- Façade Re-design for PtD
- Stick-Built vs. Infinity System
- Green Roof Addition
- Conclusion
- Acknowledgments



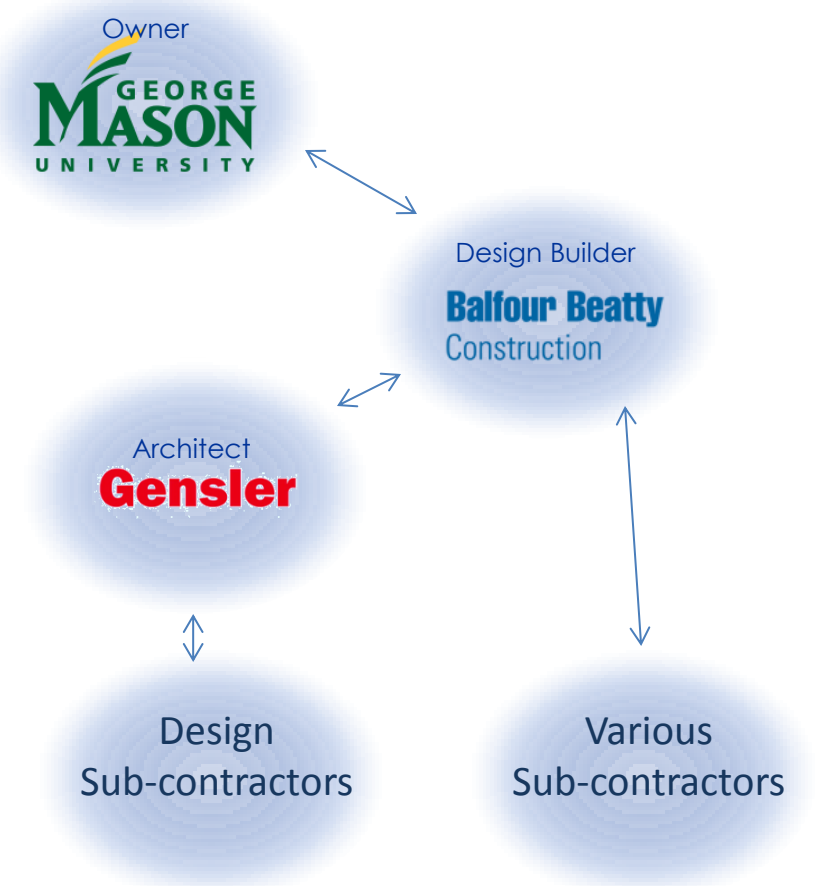
Advisor: Ed Gannon



Renderings courtesy of Genster

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Project Delivery – Design Build



Renderings courtesy of Gensler





- Project Statistics:**
- 295 Freshman Students
 - 70,057 GSF
 - ~\$16,000,00
 - 12 months of construction
 - LEED Silver



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Investigation Goals

-  Investigate new ways to increase job-site safety
-  Reduce injury risk for construction workers, future students and maintenance personnel
-  Reduce the cost of construction while maintaining quality
-  Increase the awareness for sustainable design

Preview of Analyses

- Analysis 1:** Prevention through Design
- Analysis 2:** Façade Re-design for Fall Safety
Architectural Breadth: Mechanical Access Points
- Analysis 3:** Stick-Built construction vs. Infinity System
- Analysis 4:** Green Roof Addition
Structural Breadth: Green Roof Load incurrence

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Current Industry-Wide Issues

- Lack of risk knowledge
- Lack of early collaboration/communication
- Lack of contractual obligation for safety



Balfour Beatty Construction

Prevention through Design

Critical Industry Research



“...the Center for Disease Control and Prevention (CDC) have pinpointed in recent studies that an abundant **37%** of these work related injuries are **directly caused by poor design**” - CDC.gov

Ideas for Implementation

- Raising sill height to 39”
- Prefabrication
- Raising parapet height and using flat roofs
- Lowering mechanical access points
- Using low VOC materials

A N A L Y S I S T W O

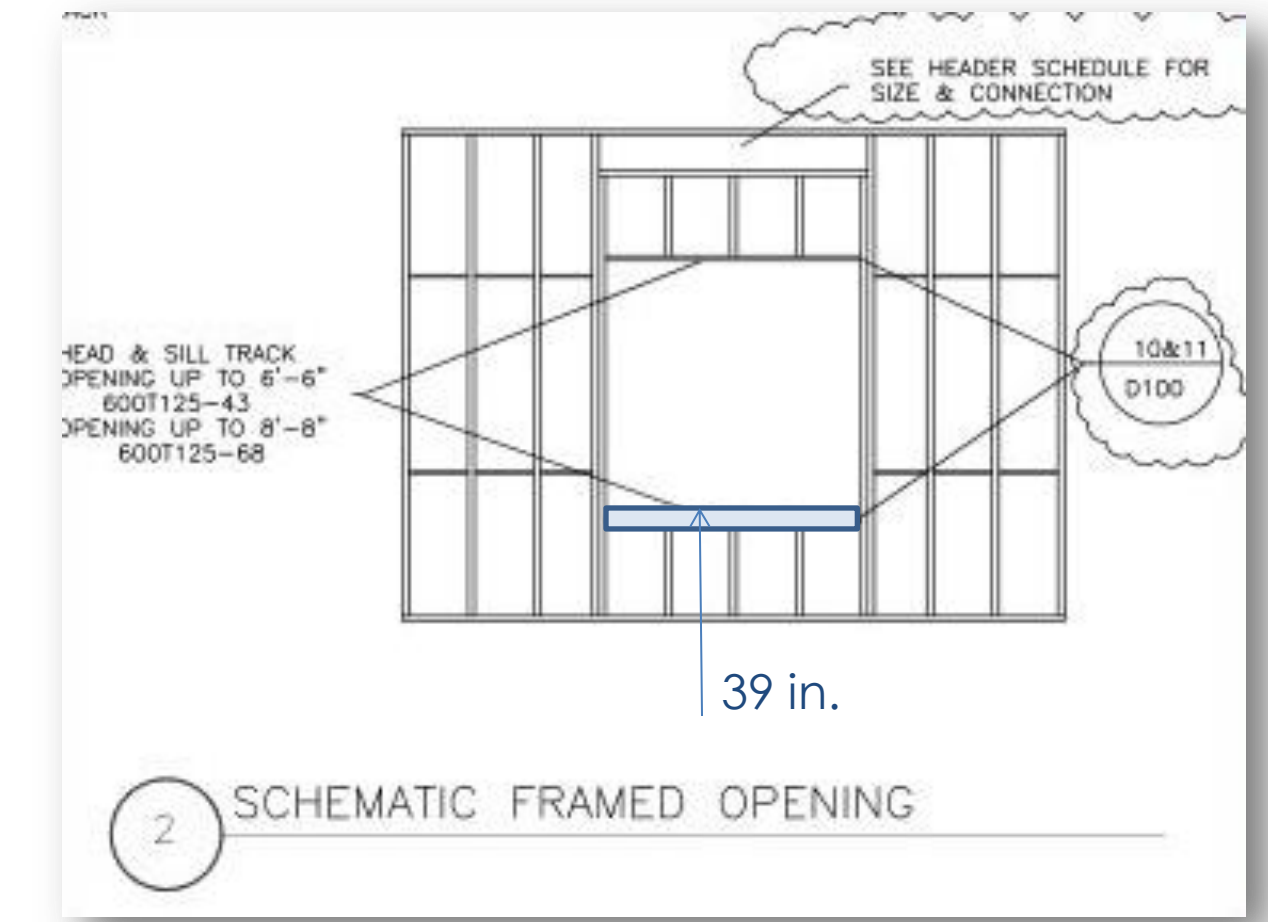
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Results:

- Increase job site safety
- Prevent student misconduct
- Benefits outweigh cost
- Save cost on temporary fall protection lines

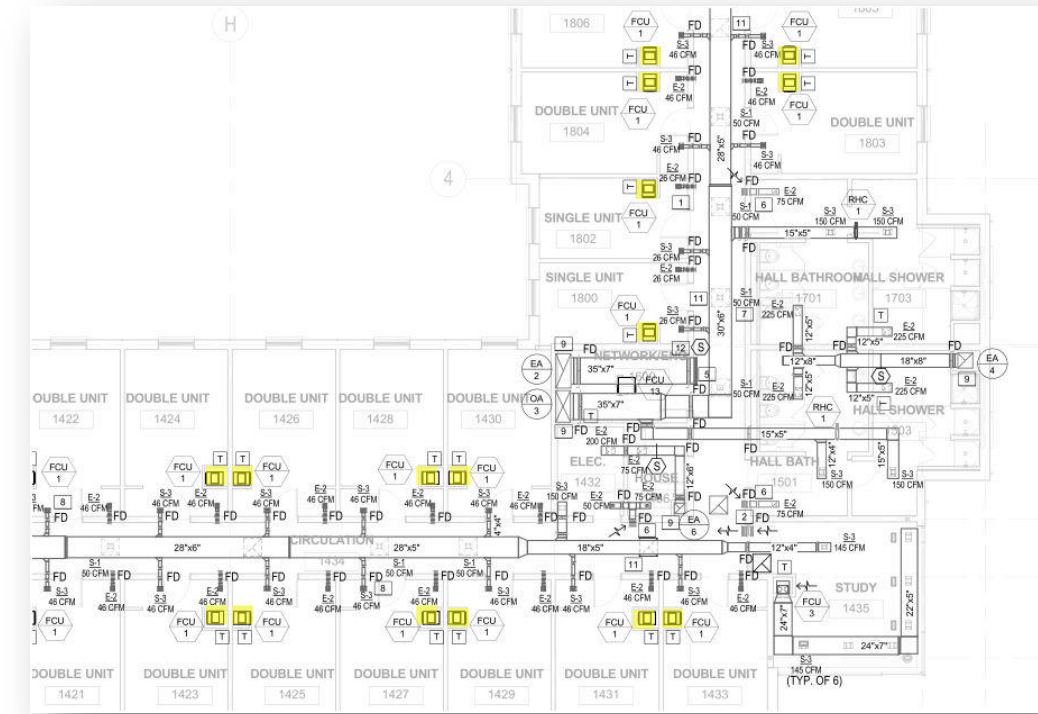
Façade Re-design and Implementation of Prevention through Design

- Logical Safety improvement method given prefab structural system
- Total cost of implementation: **\$17,156.40**
- No schedule impact



A R C H I T E C T U R A L B R E A D T H

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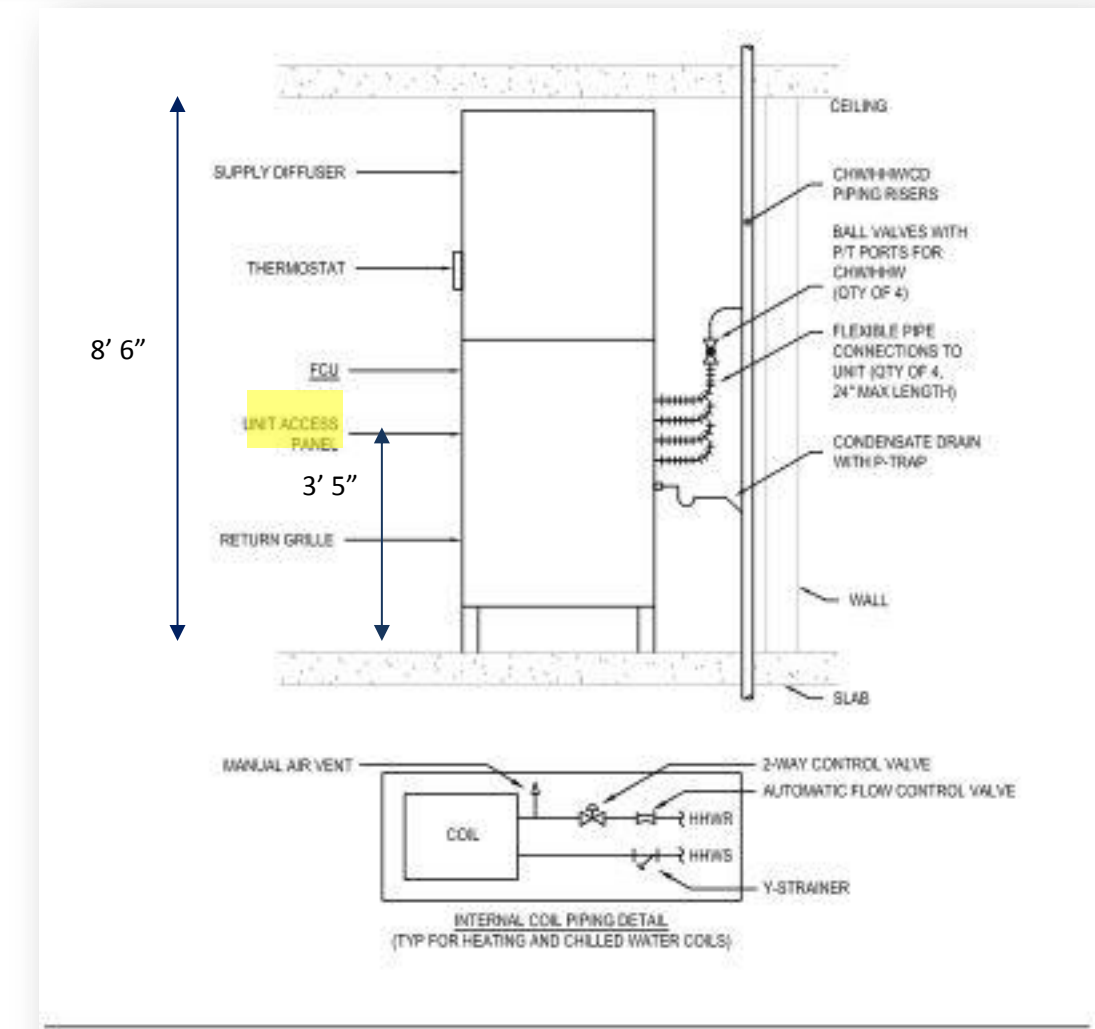


FCU Access points in each unit

Investigation of Mechanical Access Points

Mechanical Access Heights	
Mechanical Room	Floor Mounted Equipment
Unit FCU Access	3' 5"
Mechanical Penthouse	7' 8"

- All access points considered safe (except trip hazard in pent house)
- All access points behind locked doors or restricted access
- Considered very "safe" design



8 CLOSET FAN COIL DETAIL (SIDE VIEW)
 NOT TO SCALE

A N A L Y S I S T H R E E

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Benefits

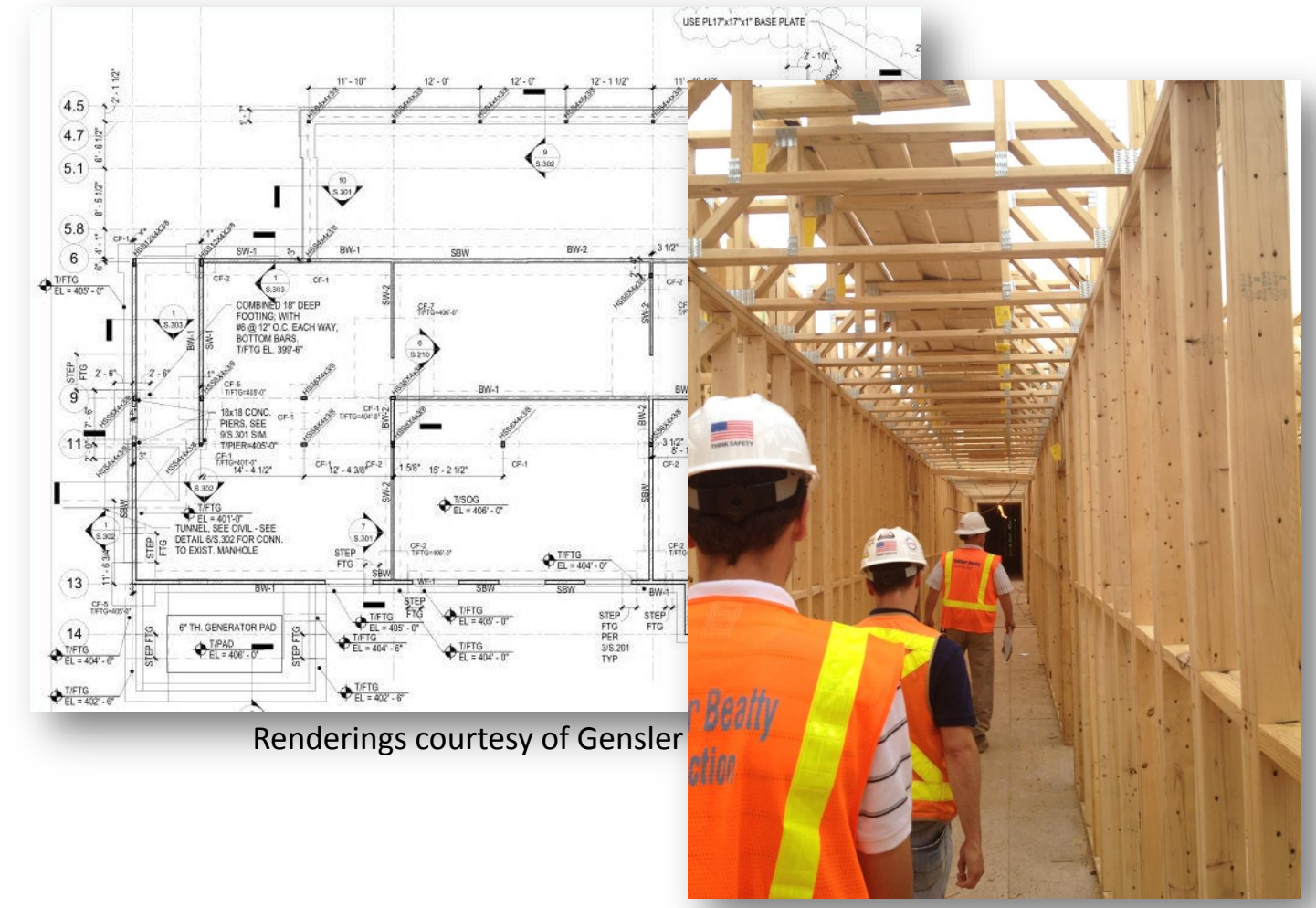
- No schedule change with prefabrication
- Fire safe construction
- Experienced craftsmen in D.C. area



Stick-Built Framing vs. Infinity Structural System

Overall Cost of Stick-Built System	
Part of System	Price
Wooden Roof Trusses/Sheathing	\$128,240.64
Wooden Framing	\$612,998.75
Wooden Joists/Girders	\$1,224,596.36
TOTAL COST	\$1,965,835.75
Total (\$/SF)	\$28.06
Location Adjustment Multiplier (78.3)	\$21.97

- Infinity Structural System estimated at **\$23 / SF**
- Total Savings of **\$72,158.71**

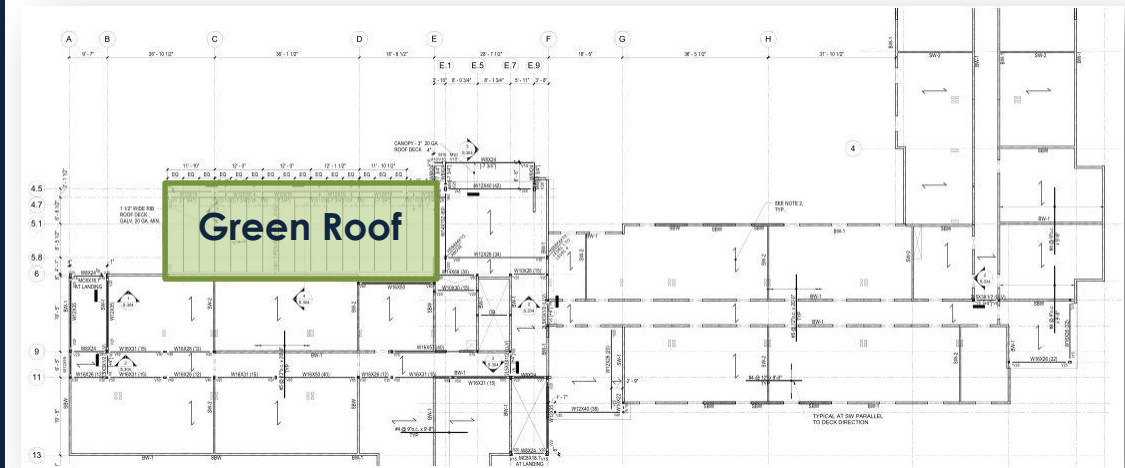


Renderings courtesy of Gensler

A N A L Y S I S F O U R

Green Roof Addition

Multi-purpose Room Green Roof



- **1,310 SF** total
- Omni-Ecosystem comprehensive style green roof for optimum weight
- Can be added post-construction
- Low maintenance, high efficiency

- Installation Cost of **\$17,696.63**
- **1 day** installation

50 – year Life Cycle Cost Benefit Analysis (\$/SF)	
Initial Cost	\$13.51
Maintenance and Replacement	\$18.25
Storm Water Reduction Savings	\$11.37
Energy Savings	\$6.37
CO2 Emission Savings	\$2.60
Community Benefits	\$30.90
Property Re-sale value premium	\$105.93

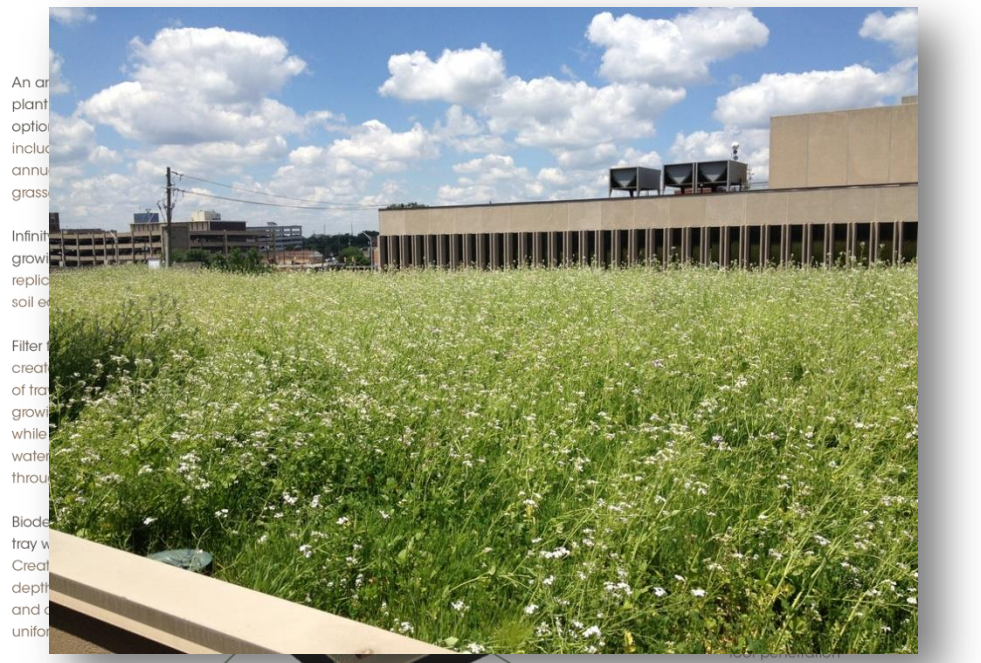
Fiscal Analysis

- Payback Period: 4.17 - 6.7 years
- Internal Rate of Return*: 4.21%
- Return on Investment*: 196%

* Not including CO2 Emissions, Community Benefits, or Re-Sale Value

Takeaways

- Significantly longer life than T.P.O.
- Educational opportunities in sustainability
- Lifetime net yield of up to **\$182,000** of benefits



An ar... plant... option... includ... annu... grass...
 Infi... grow... repli... soil e...
 Filter... creat... of tra... grow... while... water... throu...
 Biode... tray w... Creat... depth... and d... unifor...



S T R U C T U R A L B R E A D T H

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Load Comparison

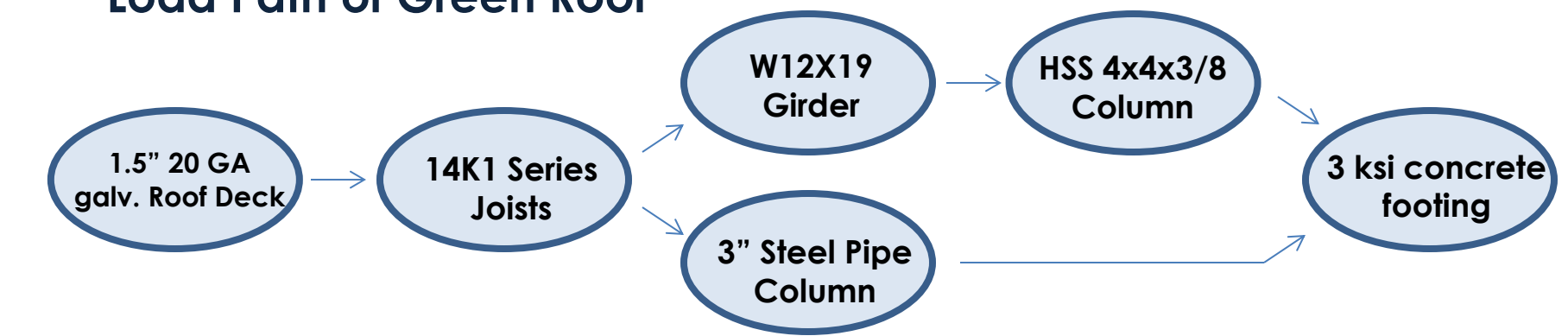
Existing Factored Load: **81 PSF**

Factored Load with Green Roof: **99 PSF**

15 PSF

Structural Evaluation of Potential Green Roof

Load Path of Green Roof



- All structural members can safely accommodate new loading
- No structural changes are necessary

Structural Change

Member	Existing Load	New Load	Allowable
Roof Deck	81 PSF	99 PSF	159 PSF
Joists	329.2 PLF	401.2 PLF	472 PLF
Girder	3.2 kips	3.9 kips	20 kips
Steel Post Column	3.2 kips	3.9 kips	16 kips
HSS Column	9.2 kips	11.3 kips	87 kips
Soil	0.56 KSF	0.71 KSF	2.0 KSF

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Conclusion

Summary of Results		
Analysis	Result	
Prevention through Design	Reduced risk of injury over the building's life	✓
Façade Re-design	More efficient fall safety measures	✓
Stick-Built Construction	Cost effective solution for the D.C. area	✓
Green Roof Addition	Educational opportunities and financial benefits	✓



Acknowledgments

- Family and Friends
- John Riskey, Contractor Safety Coordinator of PSU OPP
- Professors and Faculty of the Architectural Engineering department

Questions?

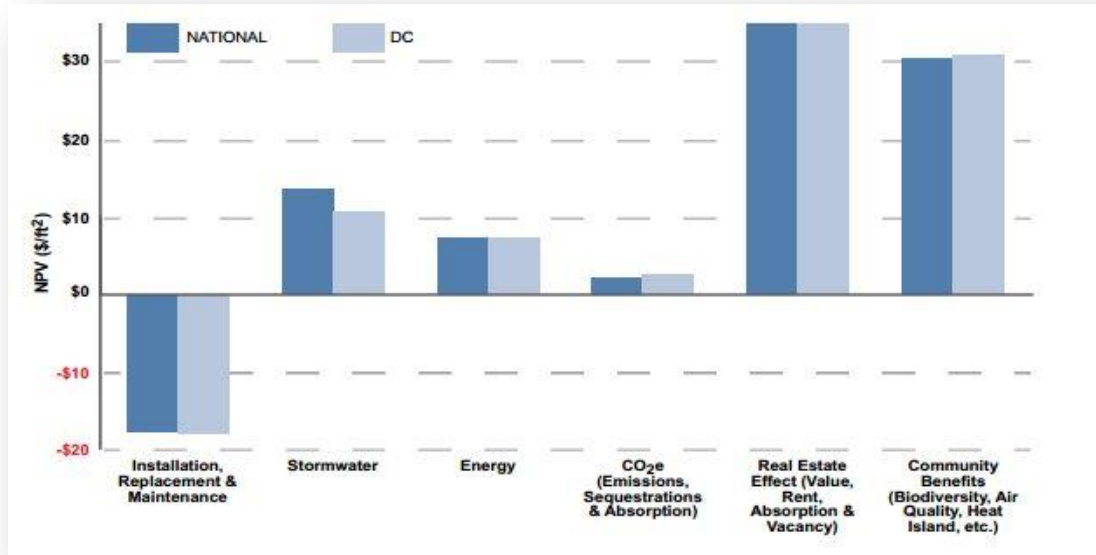


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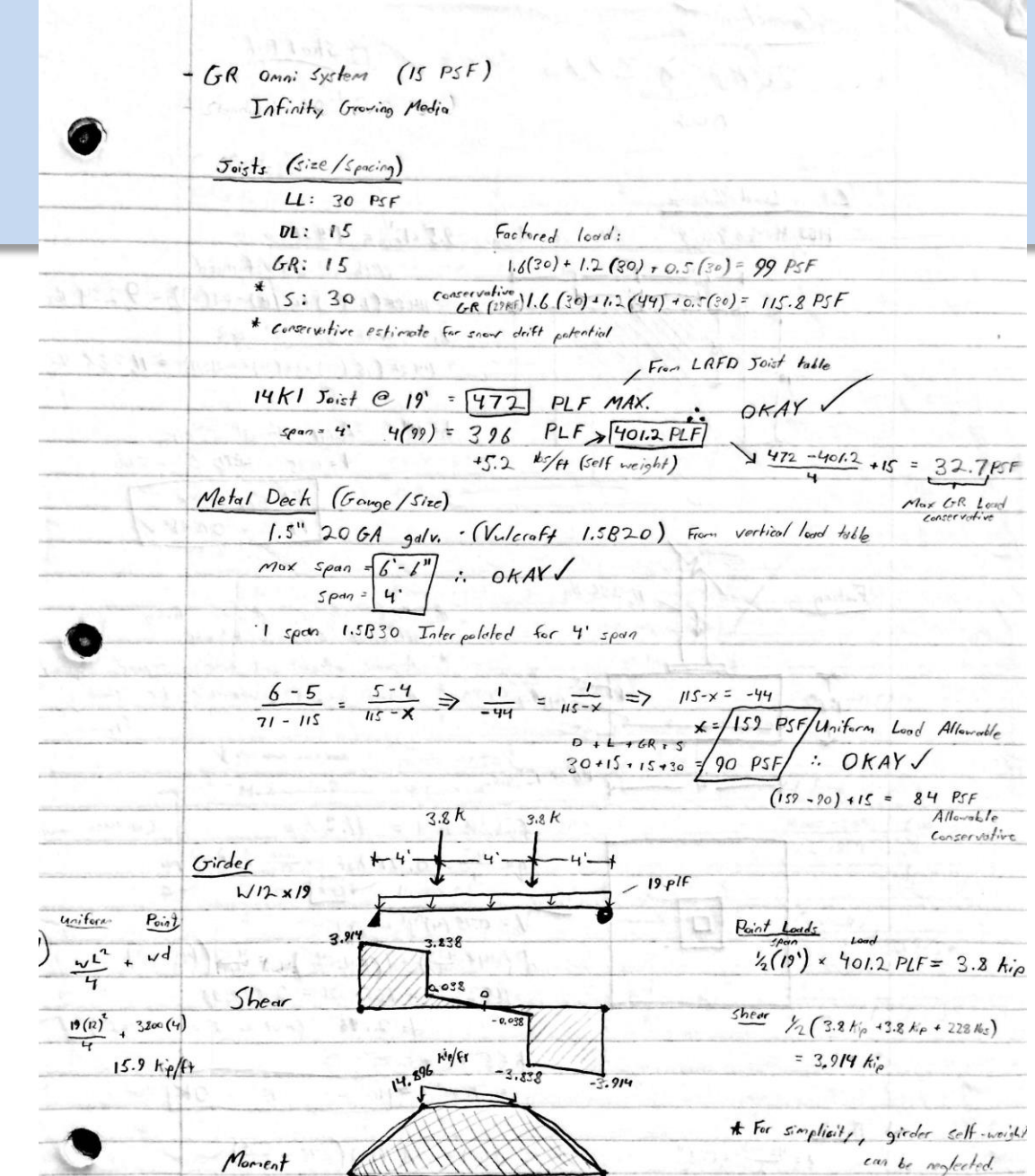
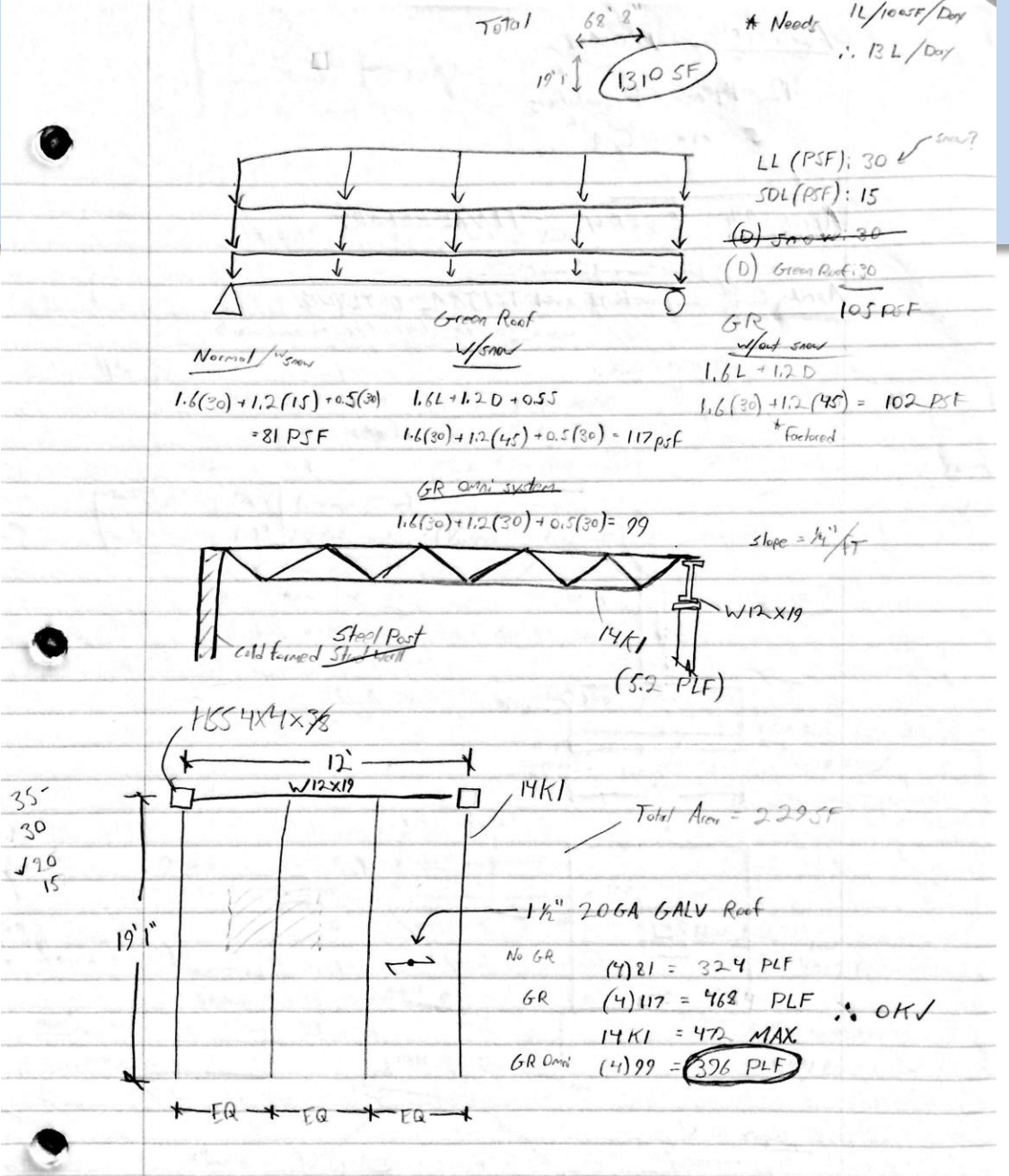


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Green Roof Study conducted by the U.S. General Services Administration



Min Deflection (δ) = $\frac{5WL^4}{384EI}$

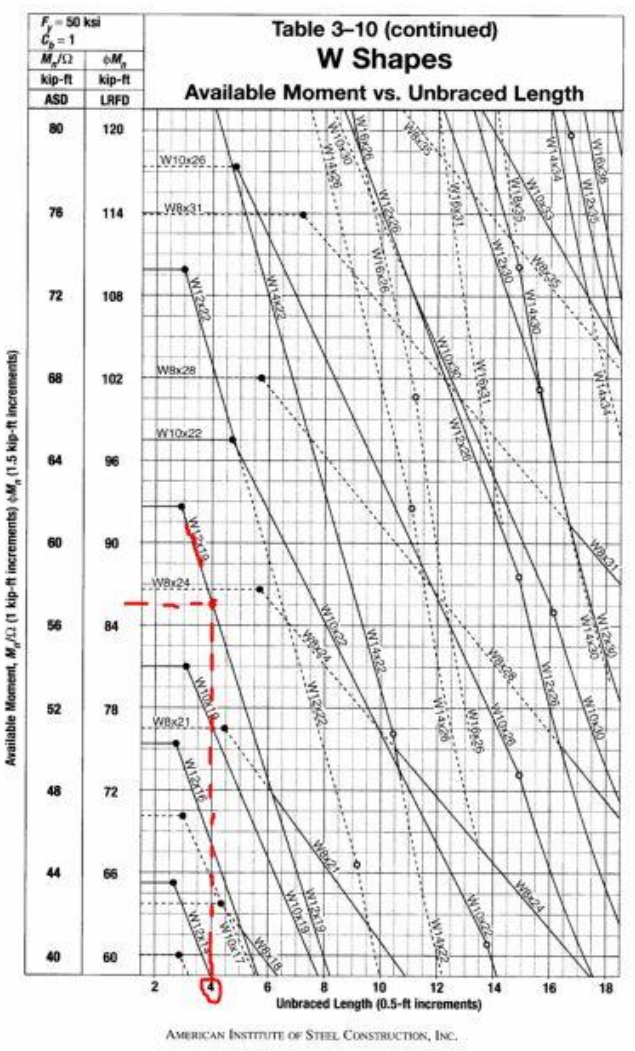
$E = 29,000,000 \text{ psi}$
 $I = 130 \text{ in}^4$
 $W = 14.8 \text{ k/ft}$
 $L = 19 \text{ ft}$

$\delta = \frac{5(14.8)(19)^4}{384(29,000,000)(130)} = 0.12 \text{ in}$

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MAXIMUM TOTAL UNIFORM LOAD TABLES 3-129



Connection
20 kip > 3.9 kip ✓ Steel Pl.
DUG

Column Load Change
HSS 4x4 x 3/8 Effective Area $9.5 \times 12 = 114.5F$

Without GR factored
 $114.5F (1.6(30) + 1.2(15) + 0.5(30)) = 9,234 \text{ lbs}$

With 2" GR
 $114.5F (1.6(30) + 1.2(30) + 0.5(30)) = 11,286 \text{ lbs}$

$h = 13'$ Axial strength in kips
 $\phi = 0.85$ LRFD Column Table
87 kips max
11.3 = OKAY ✓

Footing
11,286 lbs
1.6" x 4" x 4" #6 @ 12" o.c.

- * Since column, assume footing effective area is 4' x 4'
- * Assume adjacent soil bearing capacity $q_{ult} = 4.5$
- * Assume adjacent reinforcement for shrinkage

factored load = 11.3 kip (0.24 psf intensity)
 $q = \frac{11.3}{4} = 2.825 \text{ ksf} \times \frac{1000}{144} = 19.6 \text{ psi}$

Shear
 $V_c = 0.75(4) \sqrt{2000} = 104$
 $d^3 (164 + \frac{4.72}{4}) + d(164 + \frac{4.72}{4})4 = \frac{4.72}{4} (48 - d^3)$
 $165.23 d^3 + 665.86 d = 2,631.29$
 $d = 2.46"$ (must be 8" min so $d = 6"$)
 $h = 6" + 3" + 0.75d = 9.75"$
 $h = 10" \text{ min } 12" \therefore \text{OK}$

Flexure
 $q = \frac{48 - d^3}{2} = 17.5"$
 $M_u = \frac{4.96(17.5)^2}{2} = 5.29 \text{ k}$
 $5.29(12) = 0.9 A_s (60) (9.75 - \frac{1.2 A_s}{2})$
 $1.18 = 2.75 A_s - 0.98 A_s^2$
 $A_{s, min} = 0.12 \text{ in}^2/\text{ft}$
 $4 \text{ @ } 11" = 0.218 \text{ in}^2/\text{ft} \therefore \text{OK}$

DC Cost Data

Initial Premiums
 $\frac{10,000 - 5,000}{9.5 - 10.7} = \frac{5,000 - 1,310}{10.7 - x}$ $x = \boxed{\$11.59/SF}$

Net Present Value of Installation, Replacement & Maintenance (50-year)
 $\frac{10,000 - 5,000}{17.7 - 12.1} = \frac{5,000 - 1,310}{12.1 - x}$ $x = \boxed{\$18.25/SF}$

NPV Structure
 $\frac{10,000 - 5,000}{10.5 - 11.0} = \frac{5,000 - 1,310}{11.0 - x}$ $x = \boxed{\$11.37/SF}$

NPV Energy
 $\frac{50,000 - 10,000}{2.3 - 6.8} = \frac{10,000 - 1,310}{6.8 - x}$ $x = \boxed{\$6.47/SF}$

Internal Rate of Return
 $NPV = \sum_{t=0}^n \frac{C_t}{(1+r)^t}$
 $0 = -11.59 + \frac{-0.0072}{(1+r)^{50}}$
 $r = \boxed{4.21\%}$

Payback
 $\frac{50,000 - 10,000}{6 - 6.6} = \frac{10,000 - 1,310}{6.6 - n}$ $n = \boxed{6.7 \text{ years}}$

Return on Investment
 $\frac{50,000 - 10,550}{2.07 - 1.92} = \frac{10,000 - 1,310}{1.92 - x}$ $x = \boxed{196\%}$

Raw Estate Value
 $\frac{10,000 - 5,000}{22.2 - 22.4} = \frac{5,000 - 150}{22.4 - x}$ $x = \boxed{\$105.75/SF}$

Commitment
 $\$30.9/SF$

CO₂e
 $\$2.6/SF$

Net 50-year
Actual B₁ off
 $\frac{11.59}{2.72} = 4.17 \text{ years}$
 $\$157.27/SF$
 $-\$18.25/SF$
 $+\$137.02/SF$
 $= \$274.04/SF$

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VERTICAL LOADS FOR TYPE 1.5B

No. of Spans	Deck Type	Max. SDI Const. Span	Allowable Total (Dead + Live) Uniform Load (PSF)														
			Span (ft.-in.)											C. to C. of Support			
			5'-0"	5'-6"	6'-0"	6'-6"	7'-0"	7'-6"	8'-0"	8'-6"	9'-0"	9'-6"	10'-0"				
1	B 24	4'-8"	66	52	42	36	30	27	24	21	20						
	B 22	5'-7"	91	71	57	47	40	34	30	27	24	22	20				
	B 21	6'-0"	104	81	64	53	44	38	33	29	26	24	22				
	B 20	6'-5"	115	89	71	58	48	41	36	31	28	25	23				
	B 19	7'-1"	139	107	85	69	57	48	41	36	32	29	26				
	B 18	7'-8"	162	124	98	79	65	55	47	41	36	32	29				
2	B 24	8'-8"	206	157	123	99	81	68	58	50	44	39	34				
	B 22	8'-10"	126	104	87	74	64	55	47	41	36	32	29				
	B 21	6'-11"	102	85	71	61	52	46	40	35	32	28	26				
	B 20	7'-4"	118	97	82	70	60	52	46	41	36	33	29				
	B 19	7'-9"	132	109	91	78	67	59	51	46	41	36	33				
	B 18	8'-5"	154	127	107	91	79	69	60	53	48	43	39				
3	B 24	9'-1"	174	144	121	103	89	78	68	60	54	48	44				
	B 22	10'-3"	219	181	152	130	112	97	86	76	68	61	55				
	B 21	5'-10"	130	100	79	65	54	45	39	34	31	27	25				
	B 20	6'-11"	128	106	89	76	65	57	50	44	39	34	31				
	B 19	7'-4"	147	122	102	87	75	65	56	49	42	38	34				
	B 18	7'-9"	165	136	114	97	84	72	61	53	46	41	36				
Notes:	1. Load tables are calculated using sectional properties based on the steel design thickness shown in the Steel Deck Institute (SDI) Design Manual.																
	2. Loads shown in the shaded areas are governed by the live load deflection not in excess of 1/240 of the span. A dead load of 10 PSF has been included.																
	3. ** Acoustical Deck is not covered under Factory Mutual																

LRFD

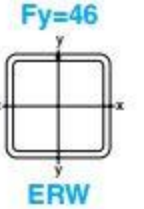
STANDARD LOAD TABLE FOR OPEN WEB STEEL JOISTS, K-SERIES
 Based on a 50 ksi Maximum Yield Strength - Loads Shown in Pounds per Linear Foot (plf)

Joist Designation	8K1	10K1	12K1	12K3	12K5	14K1	14K3	14K4	14K6	16K2	16K3	16K4	16K5	16K6	16K7	16K9
Depth (in.)	8	10	12	12	12	14	14	14	14	16	16	16	16	16	16	16
Approx. Wt (lbs./ft.)	5.1	5.0	5.0	5.7	7.1	5.2	6.0	6.7	7.7	5.5	6.3	7.0	7.5	8.1	8.6	10.0
Span (ft.)																
8	825															
9	550															
10	825	825														
11	798	825														
12	377	842														
13	666	825	825	825	825											
14	288	455	550	550	550											
15	565	718	825	825	825											
16	225	363	510	510	510											
17	486	618	750	825	825	825	825	825	825							
18	179	289	425	463	463	550	550	550	550							
19	421	537	651	814	825	766	825	825	825							
20	145	234	344	428	434	475	507	507	507							
21	369	469	570	714	825	672	825	825	825	825	825	825	825	825	825	825
22	119	192	282	351	396	390	467	467	467	550	550	550	550	550	550	550
23		415	504	630	825	592	742	825	825	768	825	825	825	825	825	825
24		159	234	291	366	324	404	443	443	488	526	526	526	526	526	526
25		369	448	561	760	528	661	795	825	684	762	825	825	825	825	825
26		134	197	245	317	272	339	397	408	409	456	490	490	490	490	490
27		331	402	502	681	472	592	712	825	612	682	820	825	825	825	825
28		113	167	207	269	230	287	336	383	347	386	452	455	455	455	455
29		298	361	453	613	426	534	642	787	552	615	739	825	825	825	825
30		97	142	177	230	197	246	287	347	297	330	386	426	426	426	426
31			327	409	555	385	483	582	712	499	556	670	754	822	825	825
32			123	153	198	170	212	248	299	255	285	333	373	405	406	406
33			298	373	505	351	439	529	648	454	505	609	687	747	825	825
34			106	132	172	147	184	215	259	222	247	289	323	351	385	385
35			271	340	462	321	402	483	592	415	462	556	627	682	760	825
36			93	116	150	128	160	188	226	194	216	252	282	307	339	363
37			249	312	423	294	367	442	543	381	424	510	576	627	697	825
38			81	101	132	113	141	165	199	170	189	221	248	269	298	346
39						270	339	408	501	351	390	469	529	576	642	771
40						100	124	145	175	150	167	195	219	238	263	311
41						249	313	376	462	324	360	433	489	532	592	711
42						88	110	129	156	133	148	173	194	211	233	276
43						231	289	349	427	300	334	402	453	493	549	658
44						79	98	115	139	119	132	155	173	188	208	246
45						214	270	324	397	279	310	373	421	459	510	612
46						70	88	103	124	106	118	138	155	168	186	220
47										259	289	348	391	427	475	570
48										95	106	124	139	151	167	198
49										241	270	324	366	399	444	532
50										86	96	112	126	137	151	178
51										226	252	304	342	373	415	498
52										78	87	101	114	124	137	161
53										213	237	285	321	349	388	466
54										71	79	92	103	112	124	147



LRFD Columns Square HSS

Design Axial Strength in kips ($\phi=0.85$)



Nominal Size	4 x 4						3 1/2 x 3 1/2					
	1/2	3/8	5/16	1/4	3/16	1/8	3/8	5/16	1/4	3/16	1/8	
Wall Thickness												
Weight Per Foot	21.63	17.27	14.83	12.21	9.42	6.46	14.72	12.70	10.51	8.15	5.61	
Design Wall Thickness	0.465	0.349	0.291	0.233	0.174	0.116	0.349	0.291	0.233	0.174	0.116	
	$F_y = 46 \text{ ksi}$											
Effective length K_L in feet	0	235	187	160	132	101	69	160	138	114	88	60
	2	231	184	158	130	99	68	156	134	111	86	59
	3	225	179	154	127	97	67	151	131	108	83	57
	4	218	174	150	123	95	65	145	125	104	80	55
	5	208	167	144	119	91	63	137	119	99	77	53
	6	198	159	137	113	87	60	128	112	93	72	50
	7	185	150	129	107	83	57	119	103	87	68	47
	8	172	140	121	101	78	54	108	95	80	62	43
	9	159	129	113	94	73	51	98	86	73	57	40
	10	145	119	104	87	67	47	87	77	65	51	36
	11	131	108	95	79	62	43	76	68	58	46	32
	12	117	97	86	72	56	40	66	60	51	41	29
	13	103	87	77	65	5						

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**Load Bearing Capacity of Standard Steel Pipe Columns
 (36 KSI Yield)**

Nominal Dia. (in.)	Outside Dia. (in.)	Wall Thickness (in.)	Weight (lb./ft.)	Allowable Load in Thousands of Pounds Unsupported Column Length (ft.)*				
				6	8	10	12	14
3	3.50	0.216	7.58	38	34	28	22	16
3-1/2	4.00	0.226	9.11	48	44	38	32	25
4	4.50	0.237	10.79	59	54	49	43	36
5	5.563	0.258	14.62	83	78	73	68	61
6	6.625	0.280	18.97	110	106	101	95	89

* The above loads are the allowable loads for a column in which the load acts downward along the longitudinal axis of the column. For other designs, such as a column with a side load consult with an engineer for the proper size. When in doubt consult with an engineer.

SOIL BEARING CAPACITY

**TABLE 1804.2
 ALLOWABLE FOUNDATION AND LATERAL PRESSURE**

Class of Materials	Allowable Foundation Pressure (psf) ^d	Lateral Bearing (psf/f below natural grade) ^d	Lateral Sliding	
			Coefficient of friction ^a	Resistance (psf) ^b
1. Crystalline bedrock	12,000	1,200	0.70	—
2. Sedimentary and foliated rock	4,000	400	0.35	—
3. Sandy gravel and/or gravel (GW and GP)	3,000	200	0.35	—
4. Sand, silty sand, clayey sand, silty gravel and clayey gravel (SW, SP, SM, SC, GM and GC)	2,000	150	0.25	—
5. Clay, sandy clay, silty clay, clayey silt, silt and sand silt (CL, ML, MH and CH)	1,500 ^c	100	—	130

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Roof: Stick-Built vs. Infinity System

From Tech II

	\$/SF	TOTAL
Total	\$1,036,434.97	\$15.50
RS Means	\$19.98	Roof = \$128,240.64
Infinity	\$23.00	Int. Frame = \$612,998.75
Original from BDC	\$30.00	Joists/Girders = \$1,224,596.36
		Subtotal = \$1,965,835.75
		Wood Work (72.3)
		= \$28.06/SF x Fairfax VA factor
		= \$21.97/SF

Stick Built

- Wood Roof

Assumed
 Wood truss 5" 12 slope, 2.4" O.C. span is 45' 10.5" + 4' 10" → 49' 10.5"
 - 44' to 60' span

Materials	Installation	Total
4.43	2.61	7.04

Slope pitch multiplier (5 in 12) = 1.082
 Roof Area (flat) = 16,820 SF
 (pitch) = 18,216 SF

Sub-Total = \$128,240.64

- Wood Partitions

5/8" Drywall/Res. channel / 2x4 @ 16" O.C. / Res. channel / 5/8" Drywall / 1/2" Fiberglass

Materials	Installation	Total
2.10	6.65	8.75

Floor Area (Net) = 70,057
 Sub-Total = \$612,998.75

- Joists/Girders

SDL LL Majority
 Residential Load = 20 40 = 60
 Strips (Net) = 20 100 = 120

Sub-Total = \$1,224,596.36

* Assume 15' x 15' Bay

Superimposed Load = 75 PSF	Girder 8x16 4x16	Joists 2x8 @ 16	Total Load
			90

Floor Area (Net) = 70,057

Materials	Installation	Total
12.60	4.88	17.48